

Equa. No.	Description	Equation	Page No.
1.1	Peak Runoff (Rational Method)	$Q = CiA$	1-13
1.2	Time of Concentration (Kirpich Equation)	$T_c = 0.0078 L^{0.77} S^{-0.385} C_F$	1-21
1.3	Critical Depth in Open Channel Flow	$A^3/T = Q^2/g$	1-31
1.4	Froude Number	$Fr = V/(gD)^{0.5}$	1-32
1.5	Manning's Equation for evaluating uniform flow in open channels	$V = \frac{1.486 R^{2/3} S^{1/2}}{n}$	1-33
1.6	Compound Bend Angle	$\gamma = 180^\circ - \text{Cos}^{-1}(-\text{cos}(\alpha) \times \text{cos}(\beta))$	1-54
1.7a	Modified Manning's Equation for evaluating gutter flow hydraulics (depth known)	$Q = 0.56(z/n) S^{1/2} d^{8/3}$	1-66
1.7b	Modified Manning's Equation for evaluating gutter flow hydraulics (spread known)	$Q = (0.56/n) S_x^{5/3} S^{1/2} T^{8/3}$	1-66
1.8	Determine pavement cross slope for a V-shaped gutter	$S_x = S_{x1} S_{x2} / (S_{x1} + S_{x2})$	1-66
1.9	Length of inlet required to intercept 100% of gutter flow	$L_a = Q_a / (Q_a / L_a)$	1-72
1.10	Ratio of frontal flow to total gutter flow for grate inlets	$E_o = Q_w / Q = 1 - (1 - W/T)^{2.7}$	1-74
1.11	Ratio of side flow to total gutter flow for grate inlets	$Q_s / Q = 1 - Q_w / Q = 1 - E_o$	1-74
1.12	Ratio of frontal flow intercepted to total frontal flow for grate inlets	$R_f = 1 - 0.09 (V - V_o)$	1-74
1.13	Ratio of side flow intercepted to total side flow for grate inlets	$R_s = 1 / [1 + (0.15V^{1.8} / S_x L^{2.3})]$	1-74
1.14	Efficiency of a grate	$E = R_f E_o + R_s (1 - E_o)$	1-75
1.15	Interception capacity of a grate on grade	$Q_i = EQ = Q[R_f E_o + R_s (1 - E_o)]$	1-75
1.16	Capacity for a grate inlet in a low point or sump, inlet acting as a weir	$Q_i = CPd^{1.5}$	1-76
1.17	Capacity for a grate inlet in a low point or sump, inlet operating as an orifice	$Q_i = CA(2gd)^{0.5}$	1-76
1.18	Length of slotted pipe inlet required to intercept 100% of gutter flow on continuous grade	$L_T = 0.6Q^{0.42} S^{0.3} (1/nS_x)^{0.6}$	1-77
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1.22	Capacity for a slotted pipe inlet (with a slot width of 1.75 in) in a low point or sump, inlet operating as an orifice	$Q_i = 0.94Ld^{0.5}$	1-78
1.23	Capacity of a slotted vane drain	$Q = Kd^{5/3}$	1-78

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1.24	Discharge rate of flow of a storm sewer	$Q = \frac{1.486 A R^{2/3} S^{1/2}}{n}$	1-81
1.25	Velocity of flow in a storm sewer flowing full	$V_{full} = \frac{0.590 D^{2/3} S^{1/2}}{N}$	1-81
1.26	Discharge rate of flow in a storm sewer flowing full	$Q_{full} = \frac{0.463 D^{8/3} S^{1/2}}{n}$	1-81
1.27	Energy losses from pipe friction in storm sewers	$S_f = [Qn/1.486 AR^{2/3}]^2$	1-82
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1.29	Velocity head losses in storm sewers	$H = K(V^2)/2g$	1-83
1.30	Terminal losses in storm sewers	$H_{tm} = (V^2)/2g$	1-83
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2.1	Shear stress on a channel	$\tau = \gamma R S$	2-27
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2.8	Riprap basin depression	$F = (F \div Y_e) \times Y_e$	2-55
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1.5	Live loads on buried conduits	$W_L = \pi W L (2P_1 + P_2) / L + 24$	1-6
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